

Sample of Excel Spreadsheet Analysis and Design of Cantilever Retaining Walls

Note:
Assume the Bearing Capacity of the Foundation, $q_{all} = 3000$ psf

DESIGN OF CANTILEVER RETAINING WALL PER ACI318-05

CANTILEVER RETAINING WALL SECTION A-A & B-B

1) Design Inputs

a) Material

Concrete grade	$f_c = 35$ Mpa	5076.32 psi
Steel grade	$f_y = 420$ Mpa	60915.85 psi
Material factor	$\phi = 0.9$	
Concrete density	$\gamma_c = 24$ kN/m ³	152.88 lb/ft ³
Soil density	$\gamma_s = 18$ kN/m ³	114.66 lb/ft ³
Angle of repose for soil	$\theta = 30$ degree	30 degree
Coefficient of active earth pressure k_a	$k_a = 0.33$ radian	0.33 radian
Coefficient of passive earth pressure k_p	$k_p = 3.00$ radian	3.00 radian

b) Geometry input

B	7.00 m	22.97 ft
b_1	1.50 m	4.92 ft
D_f	2.50 m	8.20 ft
t_s	1.50 m	4.92 ft
h_e	11.10 m	36.42 ft
a	0.00 m	0.00 ft
t_1	1.00 m	3.28 ft
t_2	0.50 m	1.64 ft
b_2	4.50 m	14.76 ft
h	12.60 m	41.34 ft
H	12.60 m	41.34 ft

c) Loading

Surcharge pressure	q = 12 kN/m ²	250.62521 lb/ft ²
--------------------	--------------------------	------------------------------

Design Summary

- F.S against Overturning = 2.51 > 1.5 Safe !!
- F.S against Sliding = 1.33 < 1.5 Not Safe!! Hence Provide shear Key
- Max. bearing pressure = 372.63 kPa < 500 kPa Hence Safe
- Wall Main reinforcement = 16 @ 25c/c
- Heel Top Main reinforcement = 20 @ 75c/c
- Toe bottom main reinforcement = 16 @ 50c/c

Page 1
Design Input

Page 2
Design Output
(External Stability)
REQUIRED

Page 3
Design Output
(Internal Stability)
NOT REQUIRED
IN THIS PROJECT

SECTION A-A & B-B

1) Design Inputs

a) Material

Concrete grade	$f_c = 35$ Mpa	5076.32 psi
Steel grade	$f_y = 420$ Mpa	60915.85 psi
Material factor	$\phi = 0.9$	
Concrete density	$\gamma_c = 24$ kN/m ³	152.88 lb/ft ³
Soil density	$\gamma_s = 18$ kN/m ³	114.66 lb/ft ³
Angle of repose for soil	$\theta = 30$ degree	30 degree
Coefficient of active earth pressure k_a	$k_a = 0.33$ radian	0.33 radian
Coefficient of passive earth pressure k_p	$k_p = 3.00$ radian	3.00 radian

c) Loading

Surcharge pressure	q = 12 kN/m ²	250.62521 lb/ft ²
--------------------	--------------------------	------------------------------

Design Summary

- F.S against Overturning = 2.51 > 1.5 Safe !!
- F.S against Sliding = 1.33 < 1.5 Not Safe!! Hence Provide shear Key
- Max. bearing pressure = 372.63 kPa < 500 kPa Hence Safe
- Wall Main reinforcement = 16 @ 25c/c
- Heel Top Main reinforcement = 20 @ 75c/c
- Toe bottom main reinforcement = 16 @ 50c/c

2) Design Output

2.1) Stability Calculation

a) Check for Overturning

Load m/c/d	Load (kN)	Distance(m)	Mo (kNm)
H1	478.3	4.2	2000
H2	50.4	6.3	317.5
Total	528.7		2318 kNm

Calculation of Restoring Moment about "O"

Load m/c/d	Load (kN)	Distance(m)	Mro(kNm)
W1	899.1	4.75	4271
W2	252	3.5	882
W3	133.2	2.25	299.7
W4	86.6	1.833	156.7
W5	54	4.75	256.5
Total	1405		5831.0 kNm

Total Overturning Moment $M_o = 2318$ kNm
Total Restoring Moment $M_{ro} = 5831.1$ kNm
Factor of Safety = $M_{ro}/M_o = 5831.1/2317.9 = 2.51 > 1.5$ Safe !!

b) Check for Sliding

Total Horizontal force = 526.7 kN
Restoring force = $m \times SW = 0.5 \times 1404.9 = 702.5$ kN
Factor Safety = $H/H_r = 526.7/702.5 = 1.33 < 1.5$ Not Safe!!
Hence Provide shear Key

c) Check for Bearing pressure

C.g of Load $x = (M_{ro}-M_o)/P = 2.501$
eccentricity $e = B/2 - x = 0.999$
 $f_{bmax} = P/A(1 + 6e/B) = 372.6$ kPa < 500 kPa
 $f_{bmin} = P/A(1 - 6e/B) = 28.78$ kPa > 0
Hence Safe

2.2) Structural Design

a) Design of Stem (vertical reinforcement - inner face (1))

Design