

MSE Wall Homework

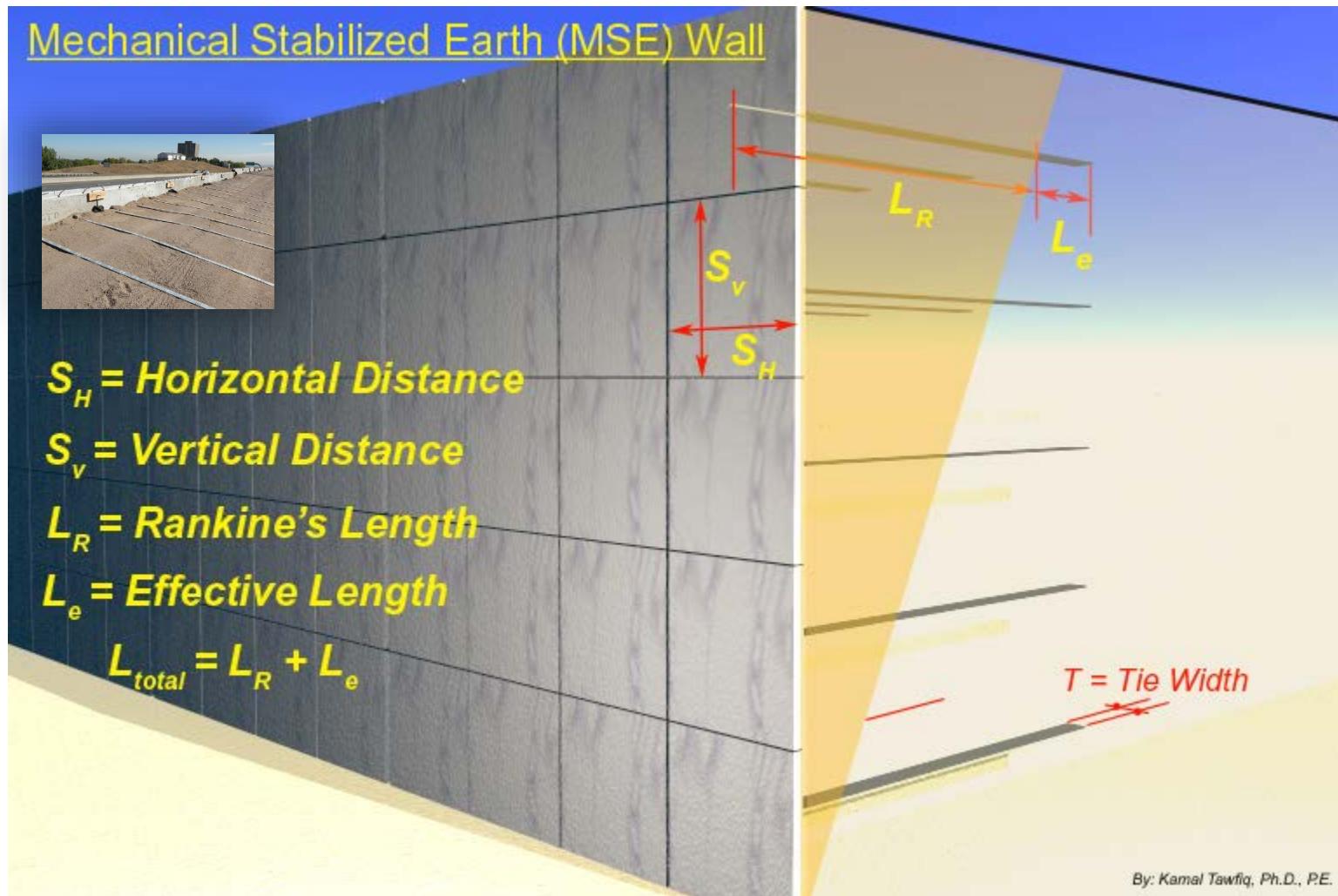
Geotechnical Design

CEG 4801

Spring 2020

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Mechanical Stabilized Earth (MSE) Wall



A reinforced earth retaining wall is to be **20 ft high**. The properties of the backfill material are $\gamma = 110 \text{ lb/ft}^3$ and $\phi = 30^\circ$. Galvanized steel ties are to be used for the construction of the wall. Design the Reinforcements with $FS_{(B)} = 1.5$, $FS_{(p)} = 1.5$, $f_y = 29,000 \text{ psi}$ and $\phi_{tie} = 20^\circ$. Take $S_v = S_h = 4'$

Mechanically Stabilized Earth (MSE) Wall

$$L_{total} = L_R + L_e$$

L_R = Rankine's Length

L_e = Effective Length

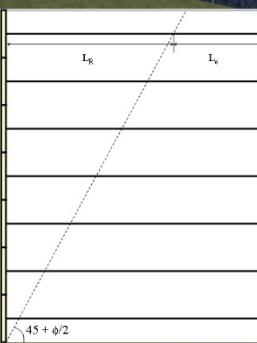
S_H = Horizontal Distance

S_v = Vertical Distance

$$\text{Tie Force} = F = \sigma_a * S_v * S_H$$

$$FS_{(tie\ Breaking)} = \frac{w t f_y}{\sigma_a * S_v * S_H} = 2.5 \text{ to } 3$$

$$FS_{(tie\ pullout)} = \frac{2 L_e w \sigma_v \tan \phi_u}{\sigma_a * S_v * S_H} = 2.5 \text{ to } 3$$



Active Wedge



Rankine's
Earth Pressure

$45 + \phi/2$

Steel Tie

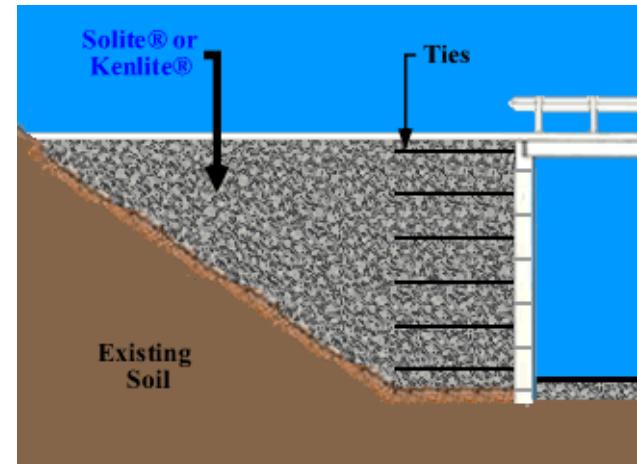
$T = \text{Tie Width}$
 $W = \text{Tie Width}$

$\gamma H k_a$

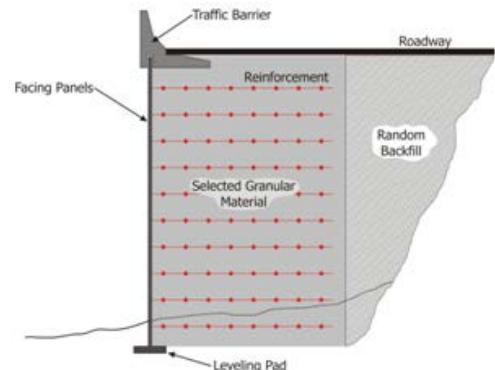
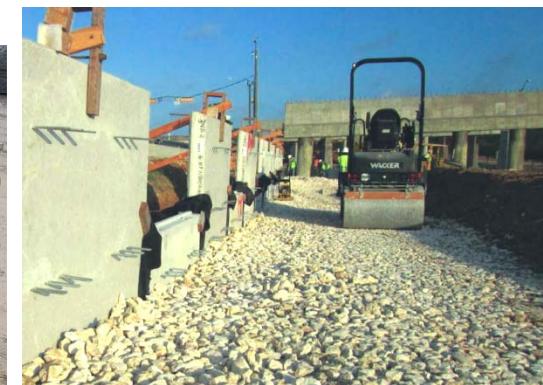
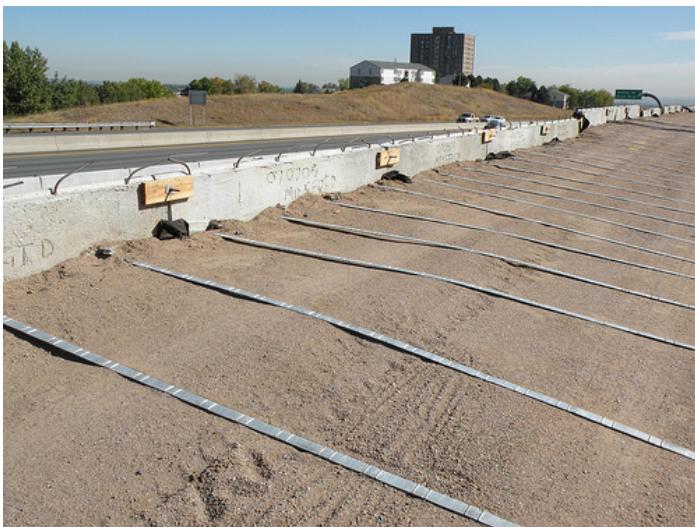
Skin Panel

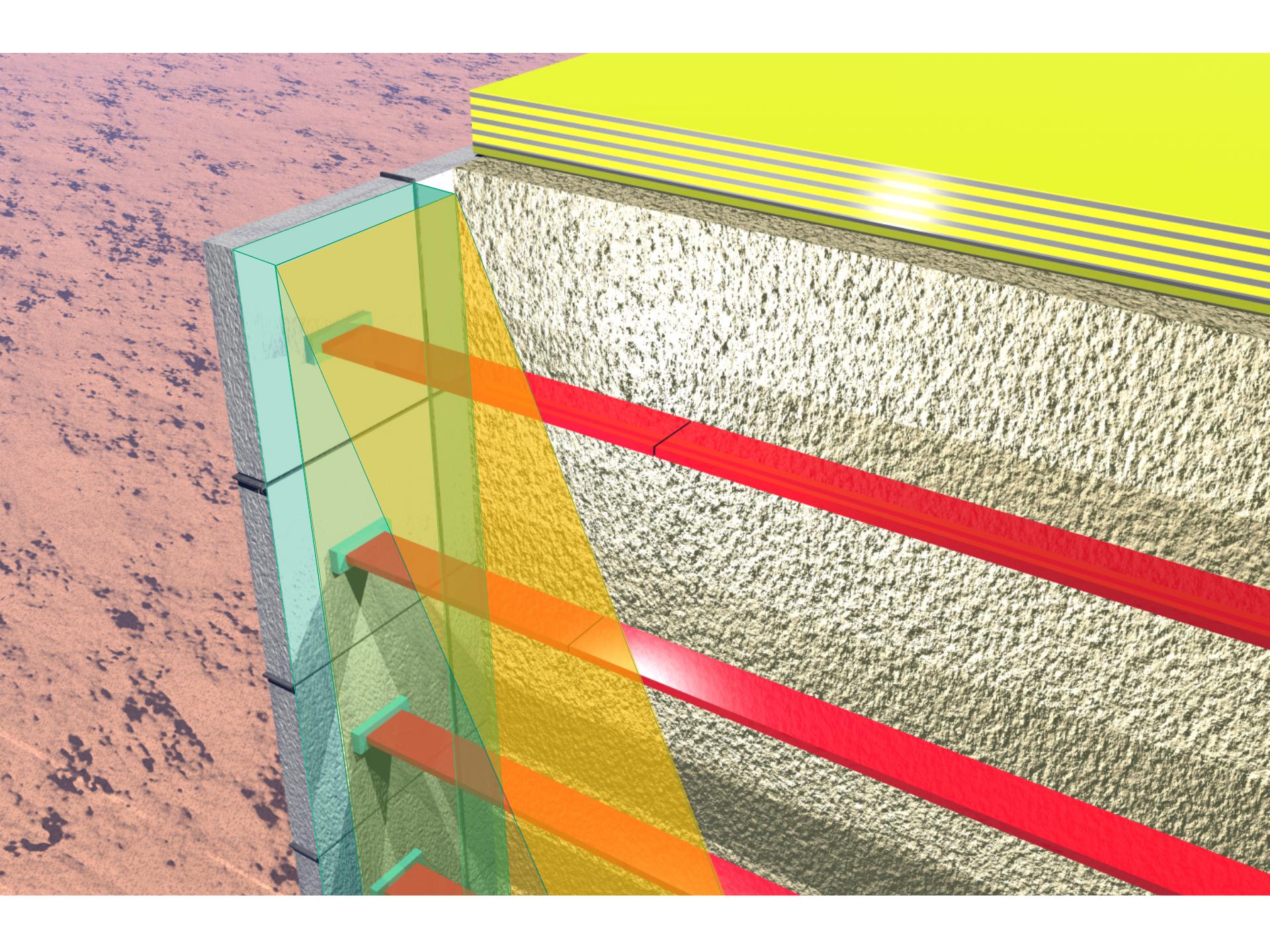
Extend the tie beyond the active wedge
to provide enough support against earth
pressure of the panel

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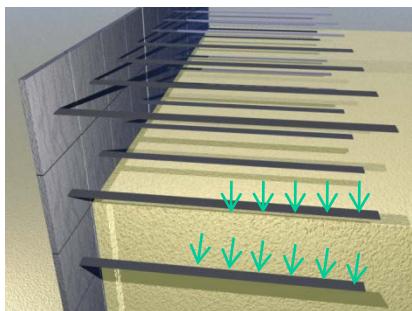




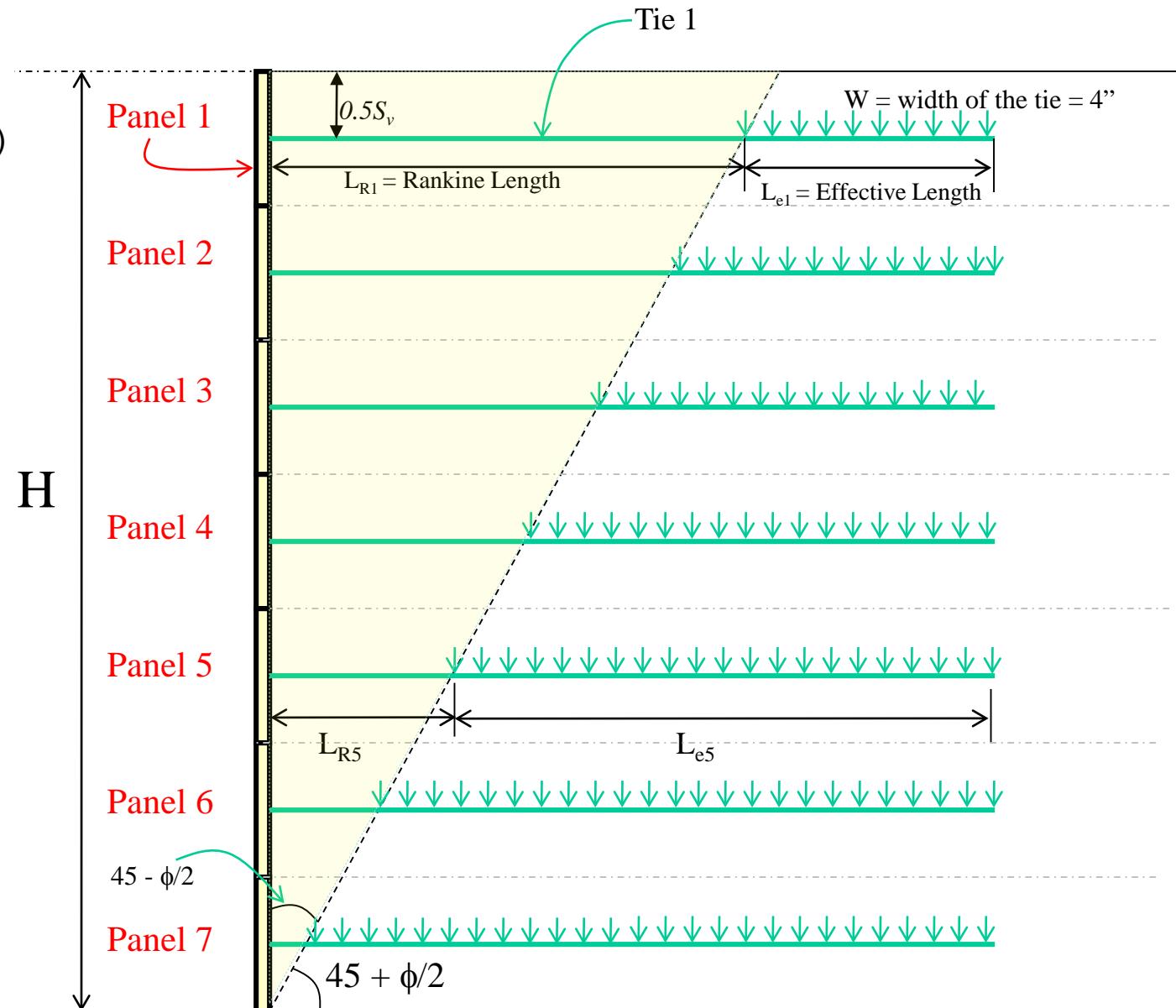
$$L_{R1} = (H - 0.5 S_v) \tan (45 - \phi/2)$$

$$L_{e1} = \frac{2.5(\sigma_a \cdot S_v \cdot S_H)}{2 w \sigma_v \tan \phi_\mu}$$

$$L_1 = L_{R1} + L_{e1}$$



$H = 30 \text{ ft}$
 $\phi = 30^\circ$
 $\gamma = 110 \text{ pcf}$

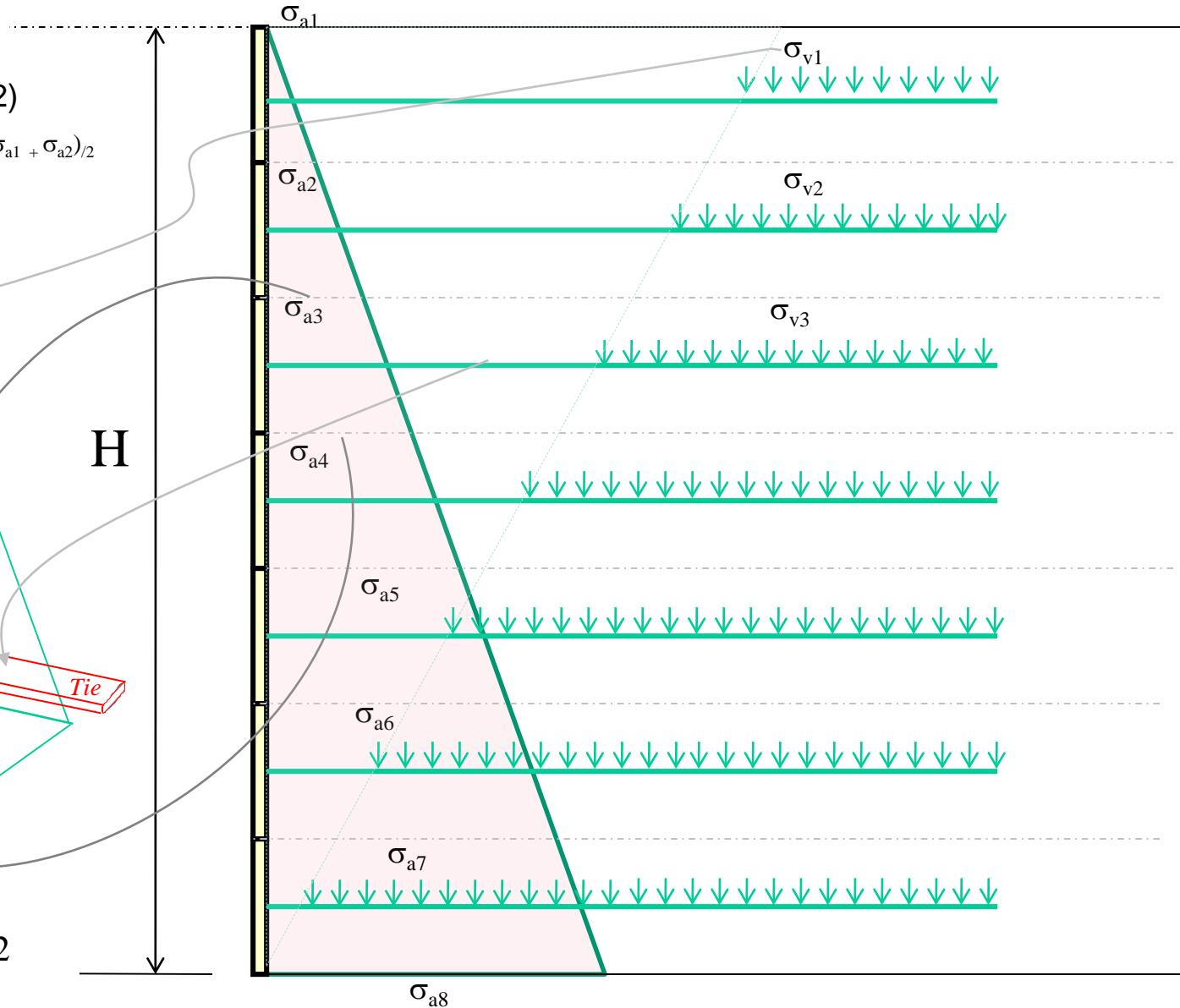
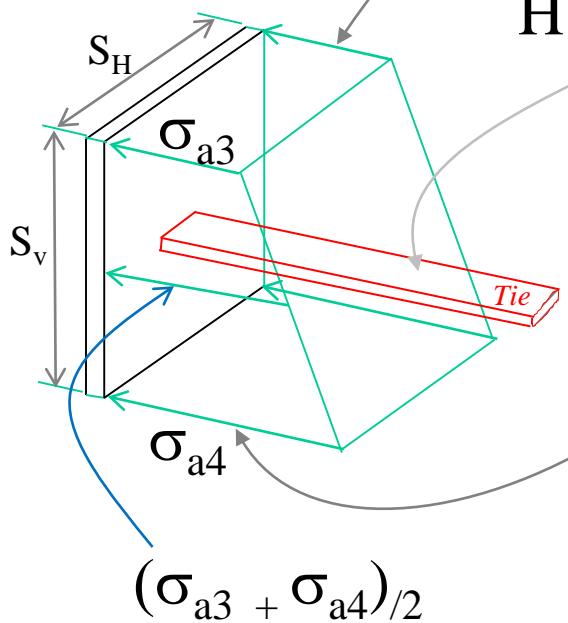


$$L_{R1} = (H - 0.5 S_v) \tan (45 - \phi/2)$$

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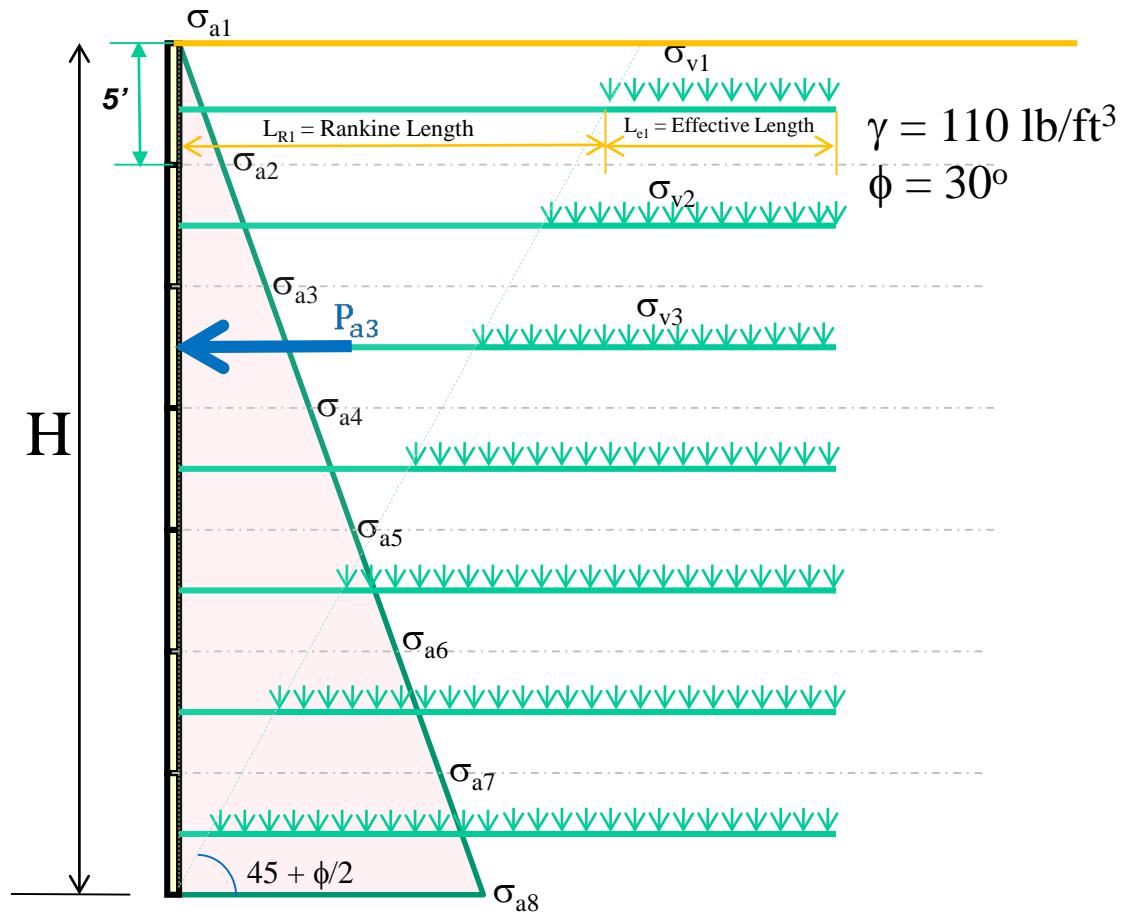
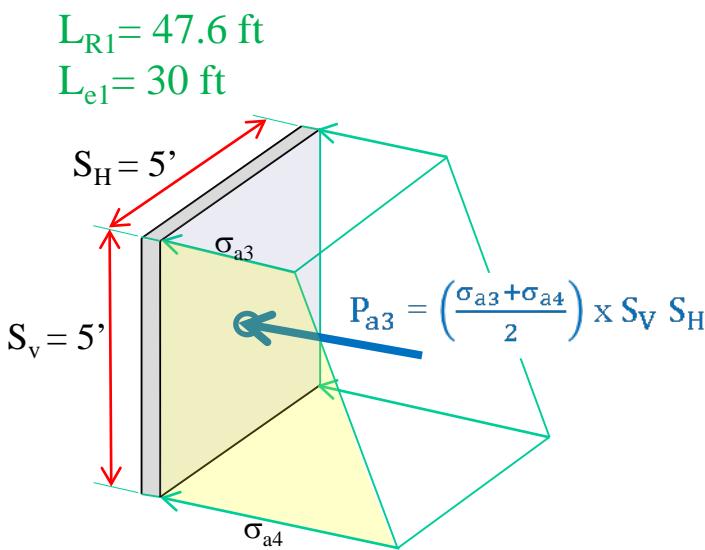
Tie width

$$L_1 = L_{R1} + L_{e1}$$



Example:

A reinforced earth retaining wall is to be **30 ft high**. The properties of the backfill material are $\gamma = 110 \text{ lb/ft}^3$ and $\phi = 30^\circ$. Galvanized steel ties are to be used for the construction of the wall. Design the Reinforcements with $FS_{(B)} = 3$, $FS_{(p)} = 3$, $f_y = 29,000 \text{ psi}$ and $\phi_{tie} = 20^\circ$. The properties of the in-situ soil below the retaining wall are $\gamma = 120 \text{ lb/ft}^3$, $\phi = 30^\circ$, and $c = 150 \text{ lb/ft}^2$. Design the panels and the ties of the wall.



Solution:

$$k_a = \frac{1 - \sin\phi}{1 + \sin\phi} = \frac{1 - \sin 30}{1 + \sin 30} = 0.33$$

Point	Tie	Depth (ft)	Vertical Stress (σ_v) lb/ft ²	Horizontal Stress (σ_a) lb/ft ²	$\sigma_a \cdot S_v \cdot S_H$	$\sigma_v \cdot \tan\phi_m$	L_e	L_R	L_{total}	$t = s_a \cdot S_v \cdot S_{H/w.fy}$
1		0	0	0						
	Tie 1	2.5	275	90.75	2268.75	100.09	102.0	47.63	150	0.058675
2		5	550	181.5						
	Tie 2	7.5	825	272.25	6806.25	300.28	102.0	38.97	141	0.176024
3		10	1100	363						
	Tie 3	12.5	1375	453.75	11343.75	500.46	102.0	30.31	132	0.293373
4		15	1650	544.5						
	Tie 4	17.5	1925	635.25	15881.25	700.64	102.0	21.65	124	0.410722
5		20	2200	726						
	Tie 5	22.5	2475	816.75	20418.75	900.83	102.0	12.99	115	0.528071
6		25	2750	907.5						
	Tie 6	27.5	3025	998.25	24956.25	1101.01	102.0	4.33	106	0.645420
7		30	3300	1089						